

# **Trackbed Structural Design**

***including Geotrack and Other Related Items***

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# ***Presentation Outline***

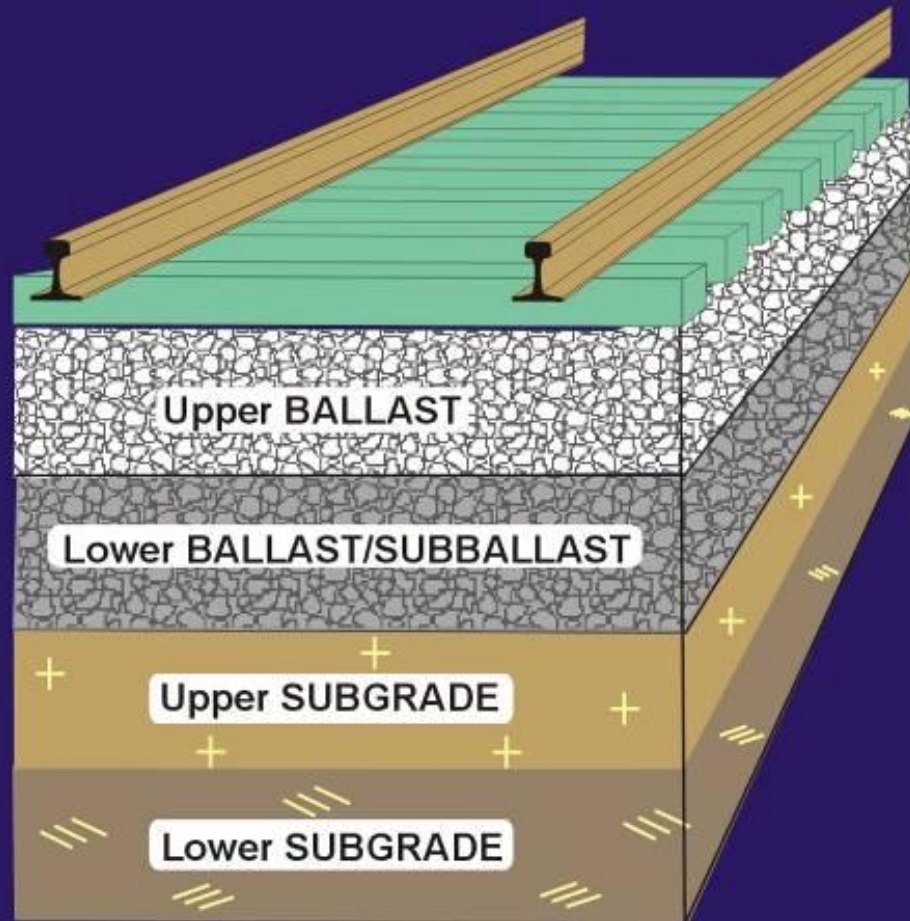
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- ◆ ***Substructure***
  - *Role & Importance*
- ◆ ***Granular Layer Thickness Design***
  - *Discussion*
  - *Example*
- ◆ ***GEOTRACK***
- ◆ ***Substructure Improvements***
  - *HMA*
  - *Reinforcement*
  - *Chemical grouting*

# **Substructure: Role & Importance**

# Track Substructure

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# ***Functions of Ballast***

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- ◆ *Resist track forces*
- ◆ *Provide resiliency*
- ◆ *Provide void storage*
- ◆ *Facilitate maintenance*
- ◆ *Provide drainage*
- ◆ *Reduce stress to underlying layers*

# ***Functions of Subballast***

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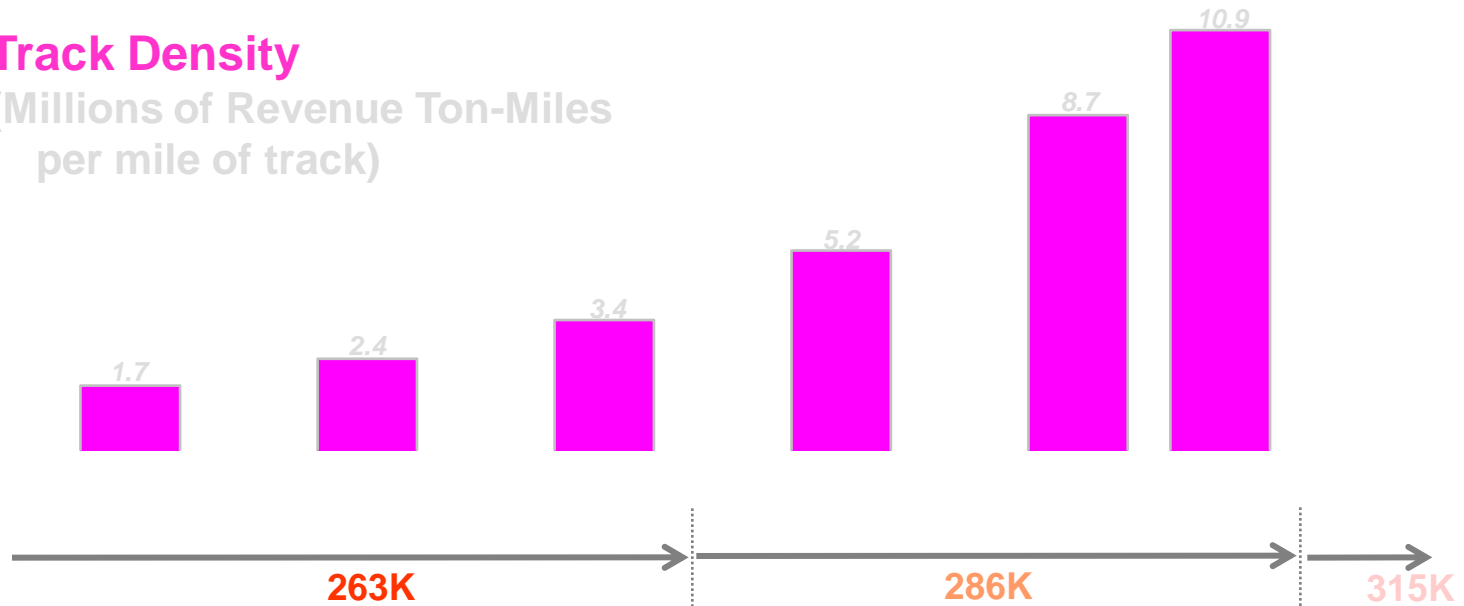
- ◆ *Reduce stress to underlying subgrade*
- ◆ *Provide frost protection*
- ◆ *Provide separation between ballast and subgrade*
- ◆ *Prevent subgrade attrition*
- ◆ *Shed water away from subgrade*
- ◆ *Provide drainage of ground water from subgrade*

# Importance of Substructure

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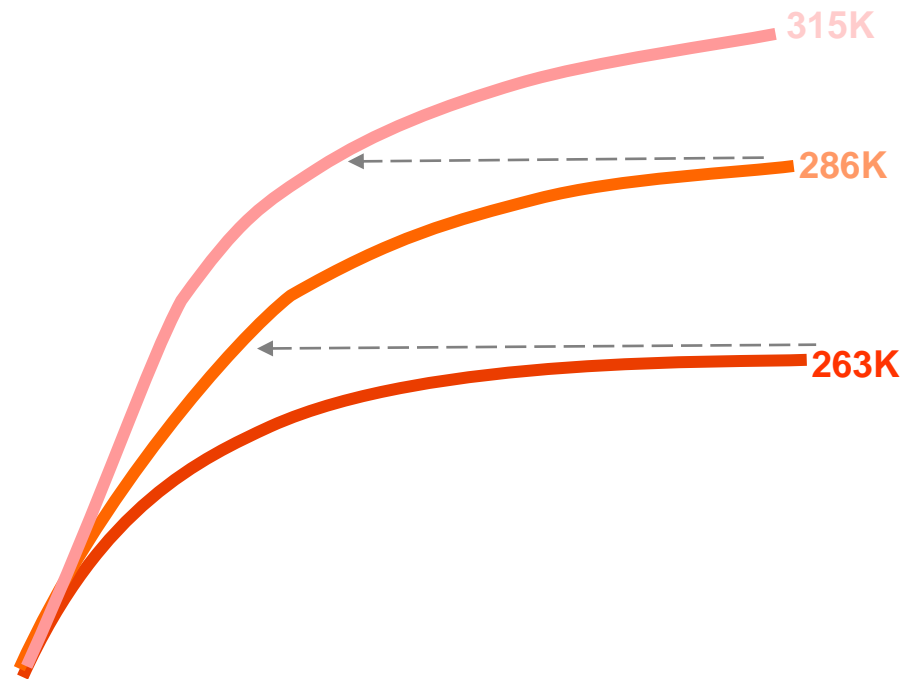
## Track Density

(Millions of Revenue Ton-Miles per mile of track)



# Importance of Substructure

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# ***Annual Substructure Related Costs***

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- ◆ ***\$14.1 Billion / yr Way & Structures***
- ◆ ***~ 50% Track Related***
- ◆ ***~ 40% Substructure Related***
  - *Direct – surfacing, cleaning, drainage, ballast*
  - *Indirect – component life, train delays*
- ◆ ***~ \$2.8 Billion/yr Substructure Related Costs***

# ***Substructure Problems***

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- ◆ *Poor drainage*
- ◆ *Fouled ballast*
- ◆ *Subgrade failure or deformation*
- ◆ *Subgrade attrition*
- ◆ *Subgrade excessive swelling and shrinking*
- ◆ *Longitudinal variation*
- ◆ *Transitions*
- ◆ *Unstable embankments*
- ◆ *Sinkholes*

# **Granular Layer Thickness Design**

# ***Granular Layer Thickness (GLT)***

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- ◆ ***GLT = Ballast + Subballast***
- ◆ ***Ballast → minimum 9 to 15 inches***
  - ***Tamping, Void Storage***
- ◆ ***Subballast → minimum 3 to 6 inches***
  - ***Drainage, Separation***
- ◆ ***GLT → minimum 12 to 21 inches***
- ◆ ***Subgrade protection ?***
  - ***Check minimum required GLT***

# ***Granular Layer Thickness Design***

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***(Ref. Li, Sussmann & Selig, 1986)***

- ◆ ***GLT = Ballast + Subballast***
- ◆ ***Limits Stress on Subgrade to Prevent :***
  - ***Progressive shear failure***
  - ***Excessive plastic deformation***
- ◆ ***Considers***
  - ***Operational conditions***
  - ***Soil conditions***

# ***GLT Design – Operating Conditions***

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## ◆ ***Traffic Parameters***

- ***Static wheel loads***
- ***Train speed***
- ***Traffic (MGT)***

## ◆ ***Converted Parameters***

- ***Design dynamic wheel load,  $P_d$***
- ***Number of repeated load cycles,  $N$***

# ***GLT Design – Material Properties***

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- ◆ ***Ballast / Subballast***
  - ***Resilient Modulus,  $E_b$***
- ◆ ***Subgrade***
  - ***Soil type***
  - ***Resilient Modulus,  $E_s$***
  - ***Compressive strength,  $\sigma_s$***
  - ***Thickness of deformable layer***

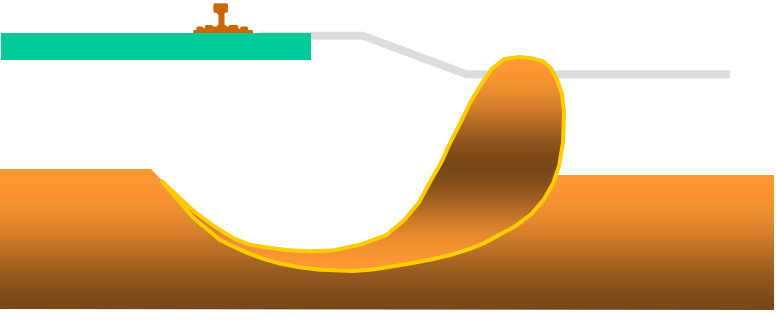
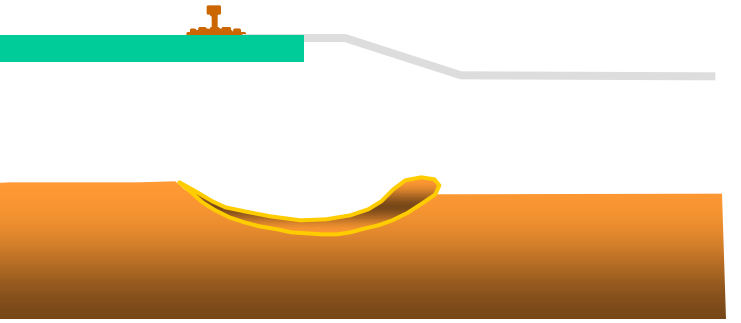
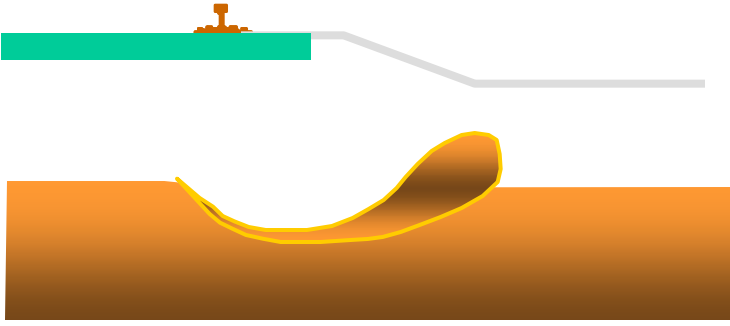
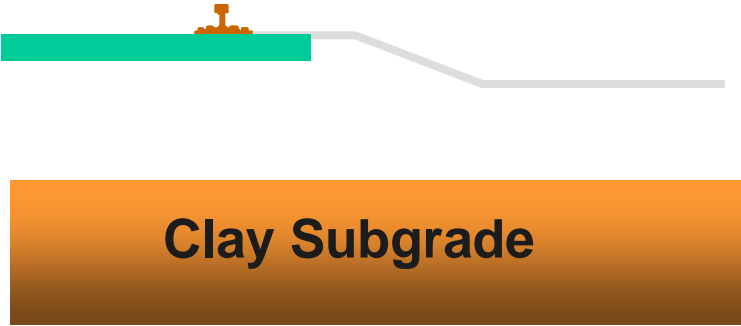
# ***GLT Design – Criteria***

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- ◆ ***Prevent subgrade progressive shear failure (Criterion 1)***
  - ***limit total cumulative plastic strain at subgrade surface for design period***
- ◆ ***Prevent excessive subgrade plastic deformation (Criterion 2)***

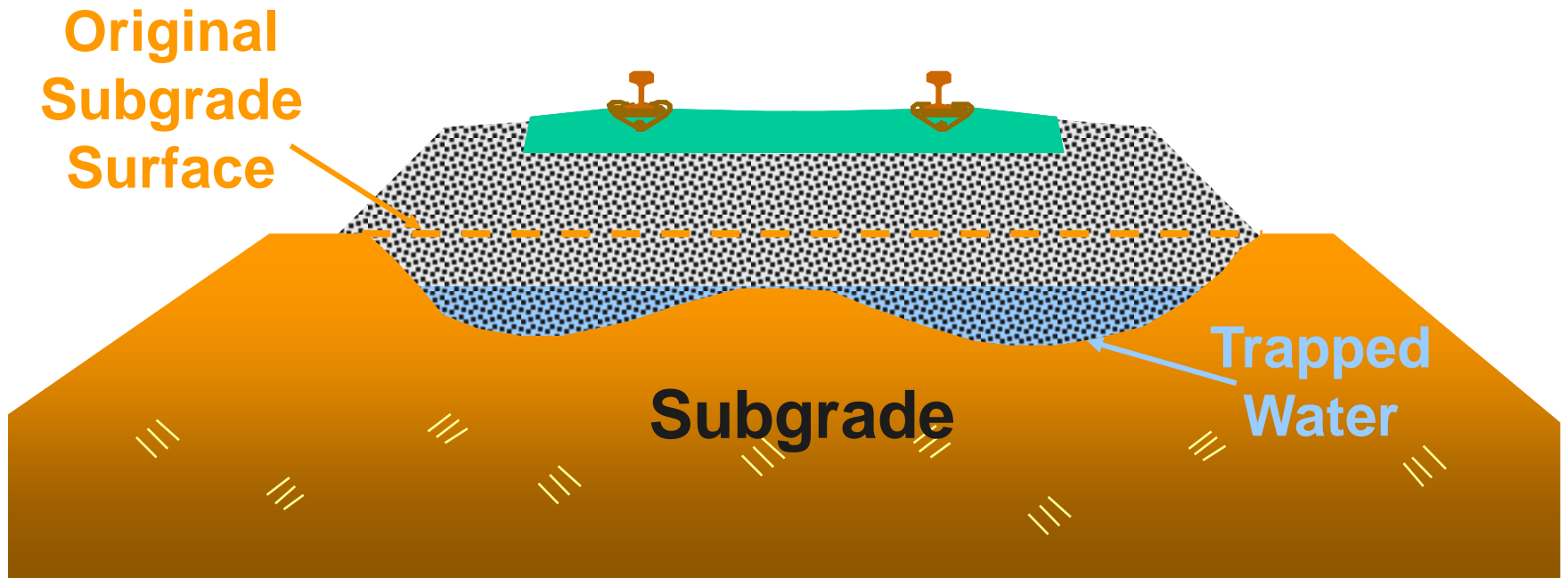
# *Progressive Shear Failure (Criterion 1)*

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# *Excessive Plastic Deformation (Criterion 2)*

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# GLT Design Example – Soil Conditions

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- ◆ **Ballast / Subballast**
  - *new, clean*
  - $E_b = 20,000 \text{ psi}$
- ◆ **Subgrade**
  - *Stiff sandy silt (ML)*
  - $\sigma_s = 21 \text{ psi}, E_s = 16,000 \text{ psi}$

Table 1: Soil Parameters for GLT Design

<b>Soil Type</b>	<b><i>a</i></b>	<b><i>b</i></b>	<b><i>m</i></b>
<i>CH</i>	<i>1.2</i>	<i>0.18</i>	<i>2.4</i>
<i>CL</i>	<i>1.1</i>	<i>0.16</i>	<i>2.0</i>
<i>MH</i>	<i>0.84</i>	<i>0.13</i>	<i>2.0</i>
<b><i>ML</i></b>	<b><i>0.64</i></b>	<b><i>0.10</i></b>	<b><i>1.7</i></b>

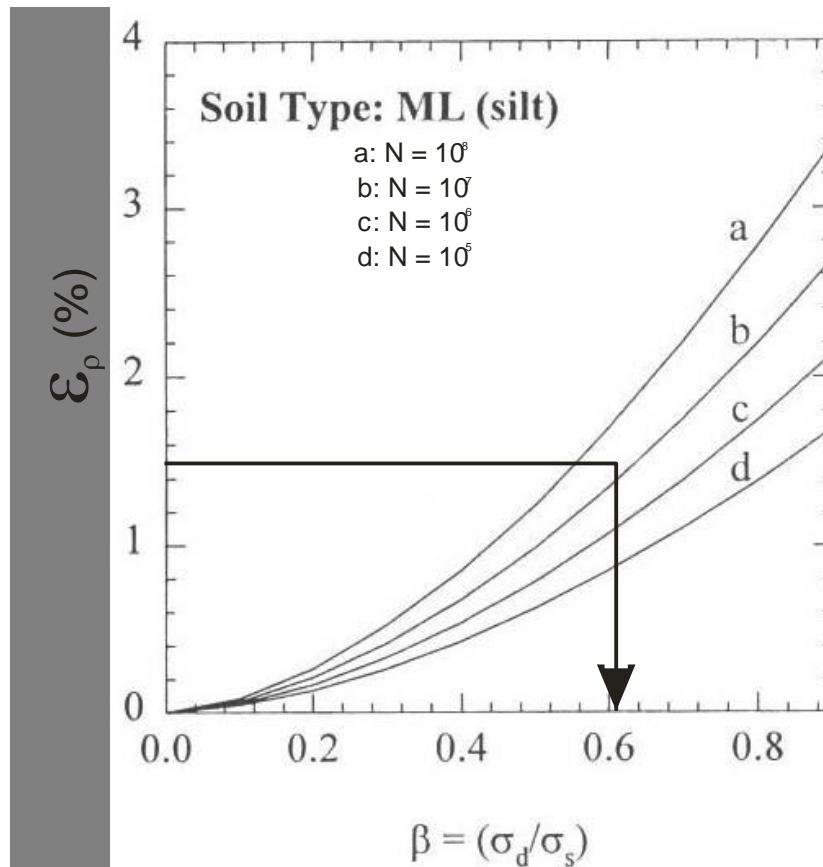
# ***GLT Example – Operational Conditions***

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- ◆ *Static wheel load,  $P_s = 18 \text{ tons} = 36,000 \text{ lb}$*
- ◆ *Dynamic wheel load,  $P_d = 58,000 \text{ lb}$* 
  - *per AREMA with 65 mph speed, 36" wheels*
- ◆ *Traffic = 100 MGT/yr*
- ◆ *Design life = 20 years*
- ◆ *Load cycles*
  - *$N = (100 \times 20) / (4 \times 18) = 2.8 \times 10^7 \text{ cycles}$*

# GLT Example – Criterion 1

- 1) Assume allowable plastic strain,  $\varepsilon_p = 1.5\%$
- 2) From 'Beta' Chart with  $\varepsilon_p$  and  $N$ :  $\beta = (\sigma_d / \sigma_s) = 0.62$



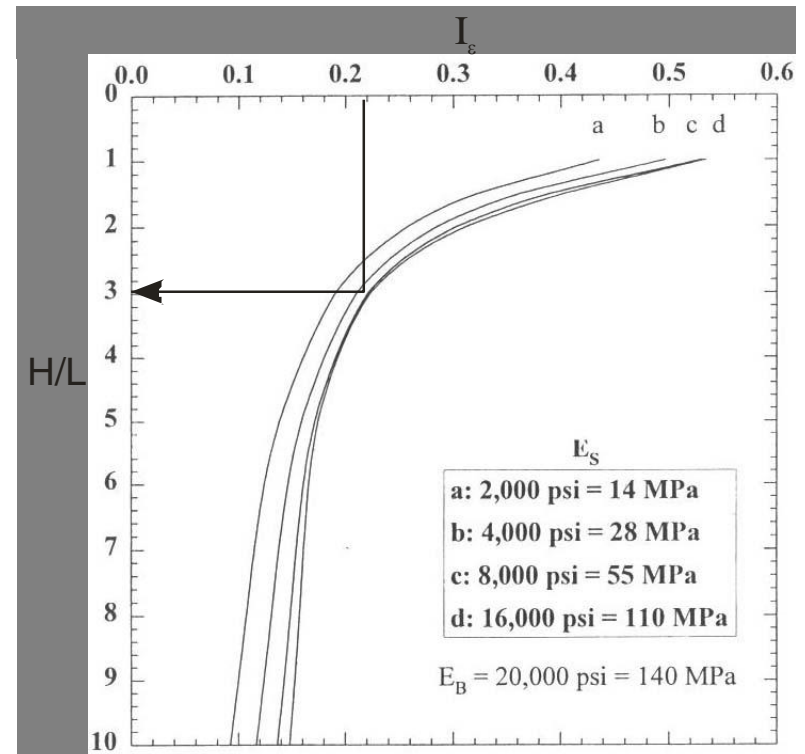
# GLT Example – Criterion 1

3)  $\sigma_d = \beta \sigma_s = 0.62 (21 \text{ psi}) = 13.0 \text{ psi}$

4) Calculate Influence Factor,  $I_\varepsilon = (\sigma_d \times A) / P_d$   
 $= (13.0 \times 1000) / 58,000 = 0.22$   
[A = 1000 for English units]

5) From Influence Chart:  $H / L = 3.0$   
[L = 6" for English units]

6)  $H = 3.0 \times 6'' = \underline{18.0 \text{ inches}}$



## ***GLT Example – Criterion 2***

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- 1) Assume allowable plastic deformation,  $\rho_a = 1$  inch*
- 2) Calculate influence factor  $I_\rho$  :*

$$I_\rho = \left( (\rho_a / L) / a (P_d / \sigma_s A)^m N^b \right) \times 100 = 0.85$$

# GLT Example – Criterion 2

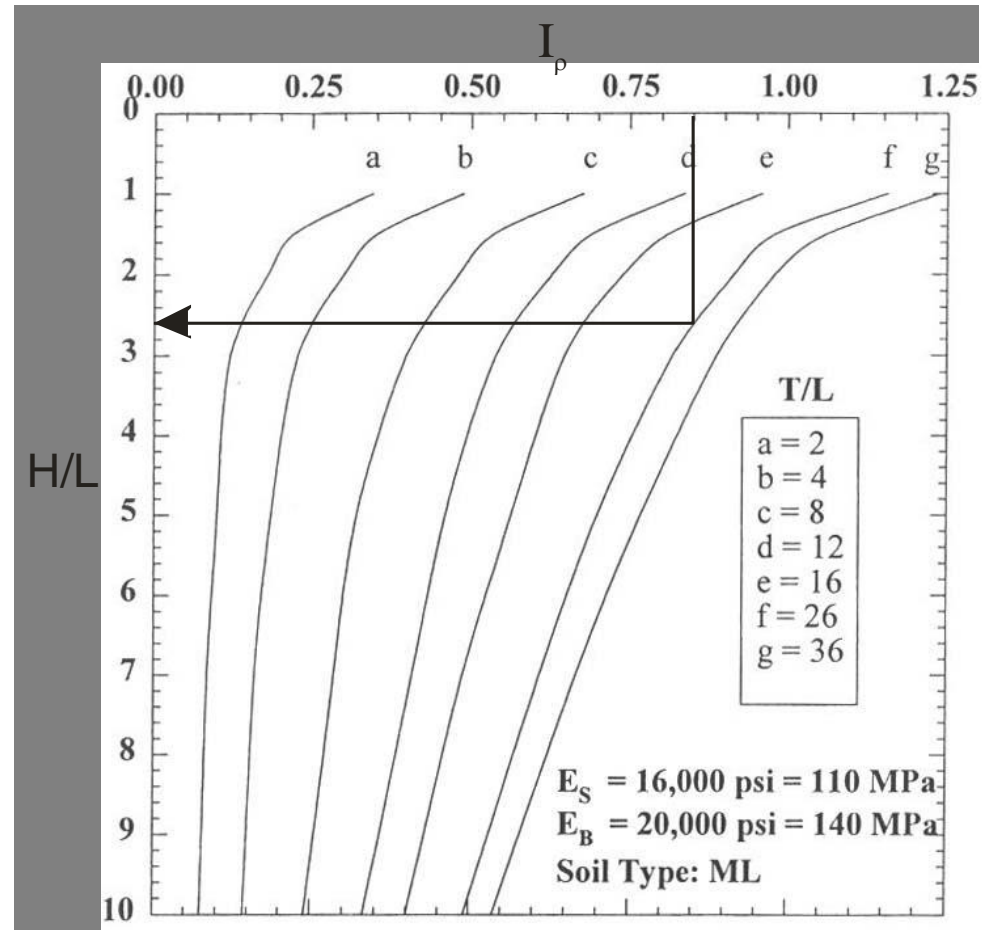
3) From Influence Chart:  $H/L = 2.6$

(Thickness of deformable layer,  $T = 13$  ft, and  $L = 6''$  for English units)

4)  $H = 2.6 \times 6'' = \underline{15.6}$  inches

Therefore,  
Criterion 1 controls:

Required GLT = 18.0 inches



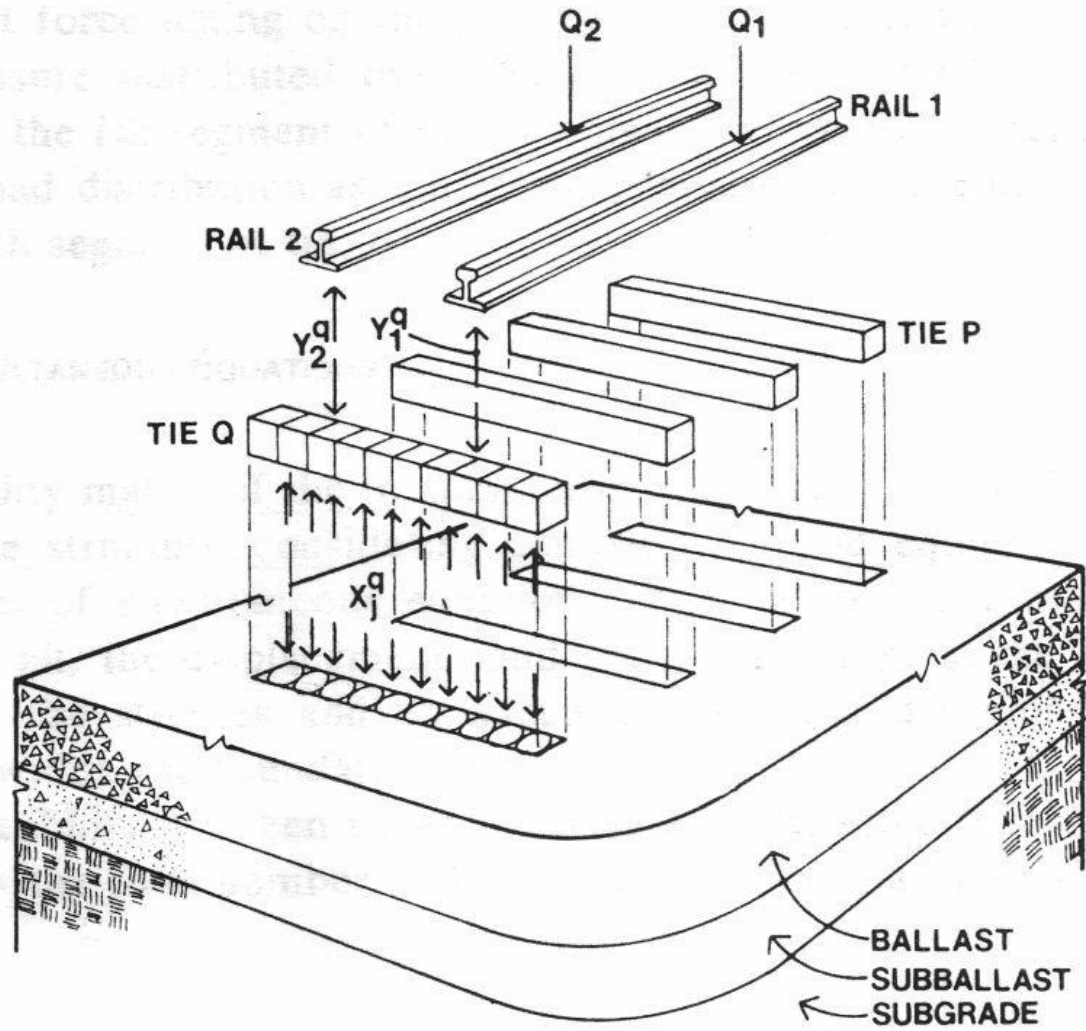
***GEOTRACK***

# ***GEOTRACK*** (Ref. Chang, Adegoke and Selig, 1980)

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- ◆ *Developed to emphasize the geotechnical aspects of track behavior*
- ◆ *Provides analysis of induced stresses and deformations in substructure*
- ◆ *3D, elastic, multilayer model*
- ◆ *Up to 4 superimposed vertical axle loads*
- ◆ *Stress-state dependent (nonlinear) modulus from iterative solution*
- ◆ *Separation of tie and ballast*

# ***GEOTRACK Forces and Elements***



# ***GEOTRACK Components***

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- ◆ *Rails – linear elastic beams with concentrated reactions at each tie*
- ◆ *Rail/Tie connection is linear spring*
- ◆ *Ties – linear elastic beams*
  - *10 equal segments*
- ◆ *Ballast, subballast, subgrade*
  - *Up to 5 linear elastic layers*
  - *infinite width - horizontal*
  - *semi-infinite half-space - vertical*

# ***GEOTRACK Output***

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## ◆ ***Superstructure***

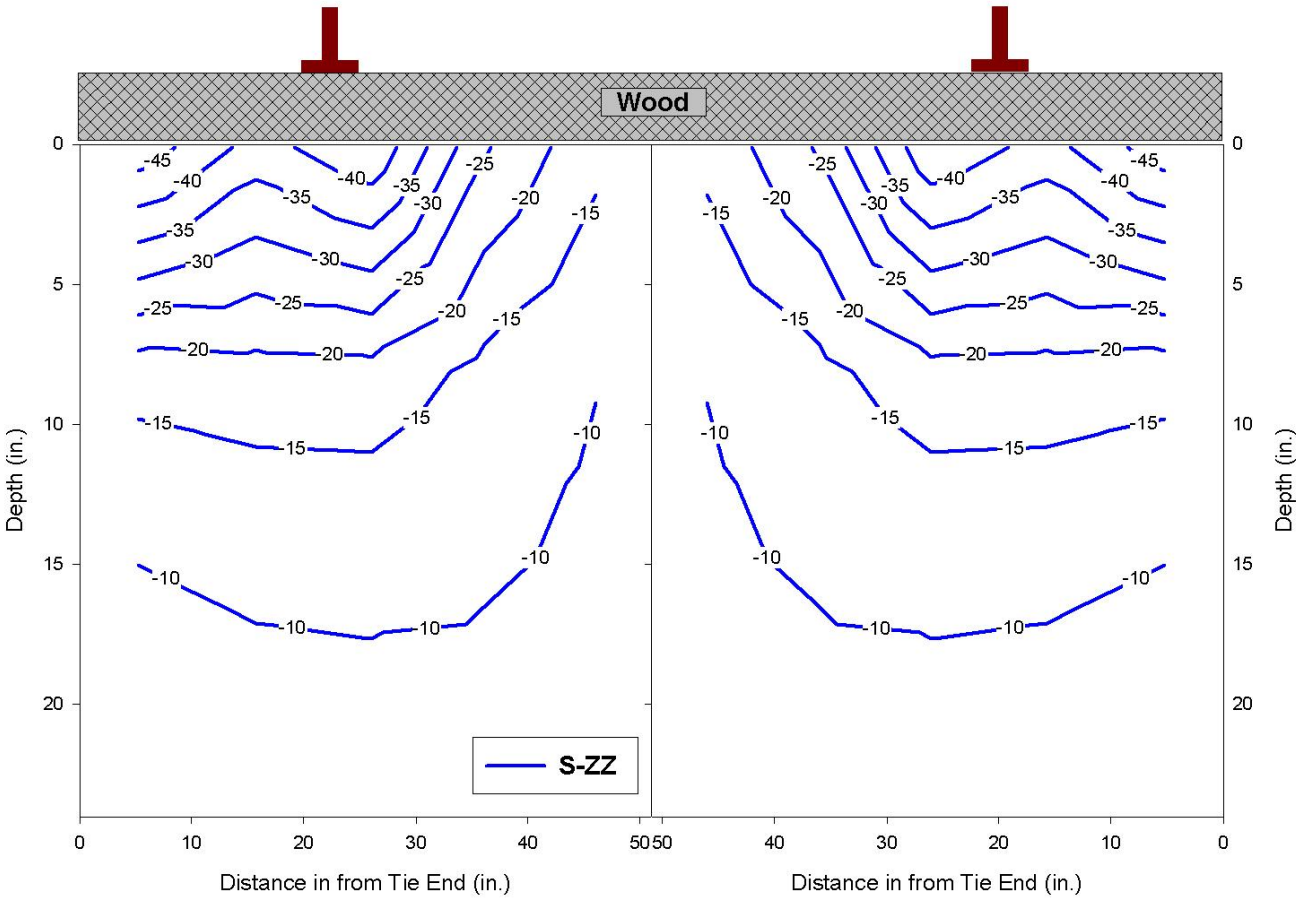
- ***Rail and tie deflections***
- ***Rail seat load***
- ***Rail and tie bending moments***
- ***Tie / Ballast reaction***

## ◆ ***Substructure***

- ***Vertical deflection***
- ***Complete stress state***
  - ***Deviator stress***
  - ***Bulk stress***
  - ***Principal stresses and directions***

## ◆ ***Track modulus for the combined system***

# Sample Output



# **Substructure Improvements**

# ***Substructure Improvements***

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- ◆ *HMA*
- ◆ *Reinforcement*
- ◆ *Chemical grouting*

# *Hot Mixed Asphalt (HMA) Underlayment*

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# *HMA Underlayment*

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## ◆ *Potential Benefits*

- *Internal drainage*
- *Stress reduction to subgrade*
- *Increase track stiffness*
- *Improved constructability of new track*

## ◆ *Key Design criteria*

- *Excessive Deformation*
  - *Accumulation of plastic strain*
- *Fatigue*
  - *Repeated load build-up of tensile strain and stress*

# HMA Underlayment

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## ◆ *Potential Benefits*

- *Internal drainage*
- *Stress reduction to subgrade*
- *Increase track stiffness... (too high?)*
- *Improved constructability of new track*

## ◆ *Key Design criteria*

- *Excessive deformation – (Accumulation of plastic strain)*
- *Fatigue – (Repeated load build-up of tensile strain and stress)*

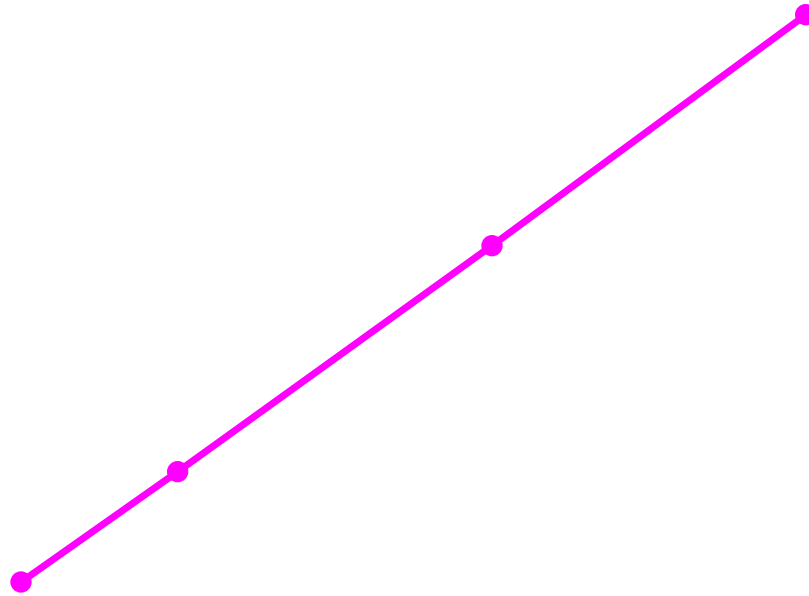
## ◆ *Key Construction criteria*

- *Good subgrade*
- *Good internal and external drainage*
- *Compaction (low voids, not  $<175^\circ$ )*

# Predicted Life of HMA

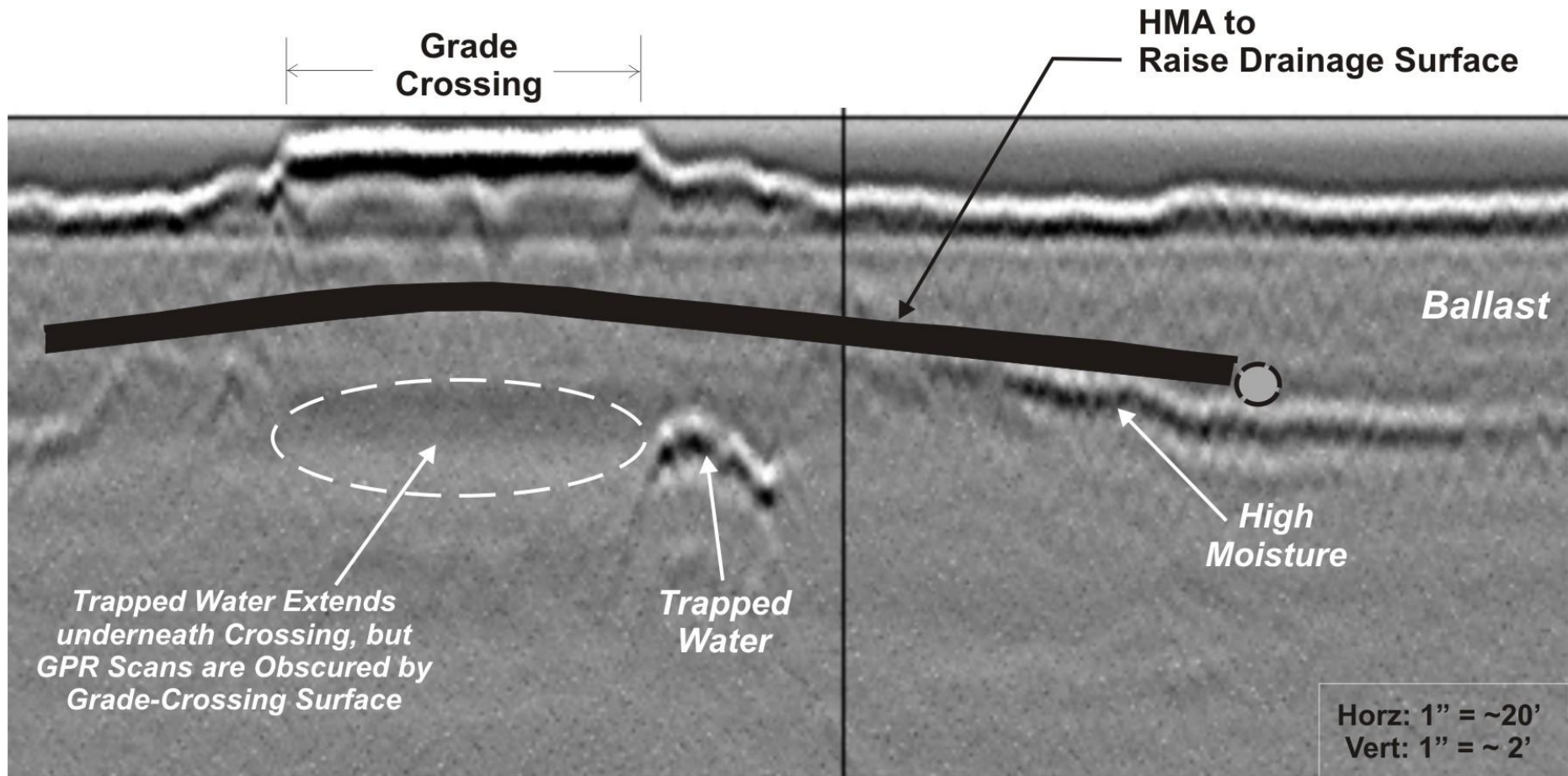
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12" Ballast, 6" HMA,  $E_a = 500,000$  psi, 36K Wheel Load



*(Based on Huang et al, 1987 and Rose, 1987)*

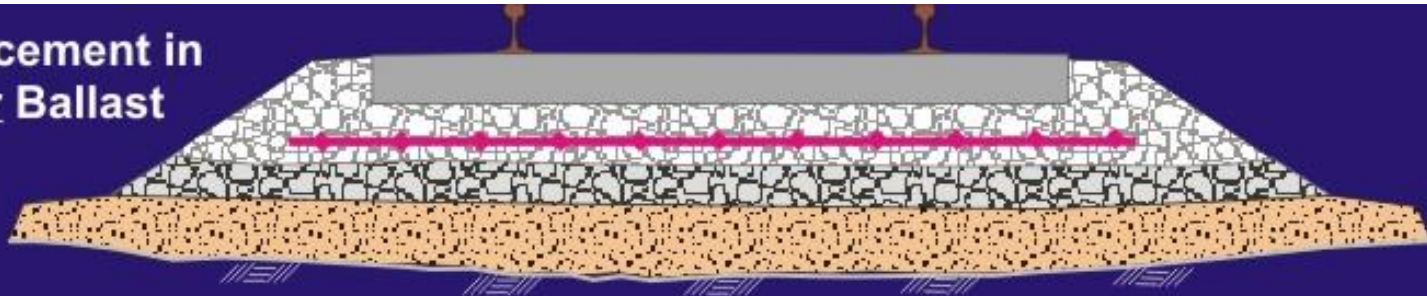
# Example: Grade Crossing – GPR and HMA



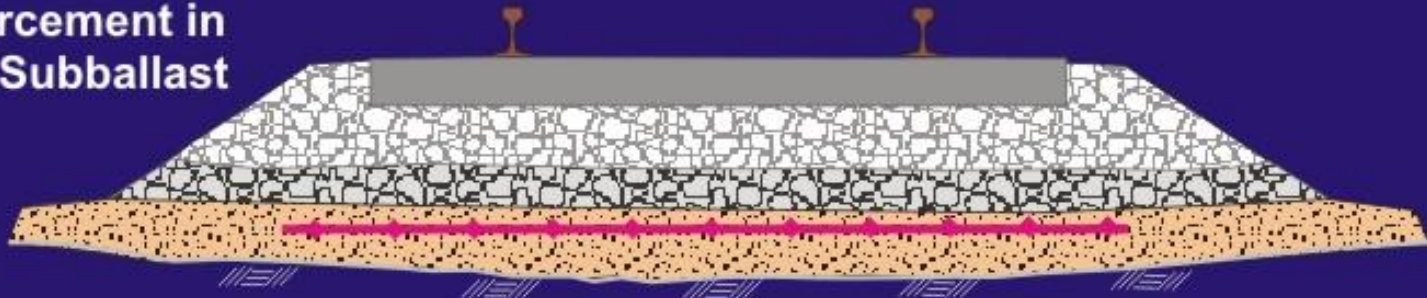
# Reinforced Ballast

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Reinforcement in  
Upper Ballast



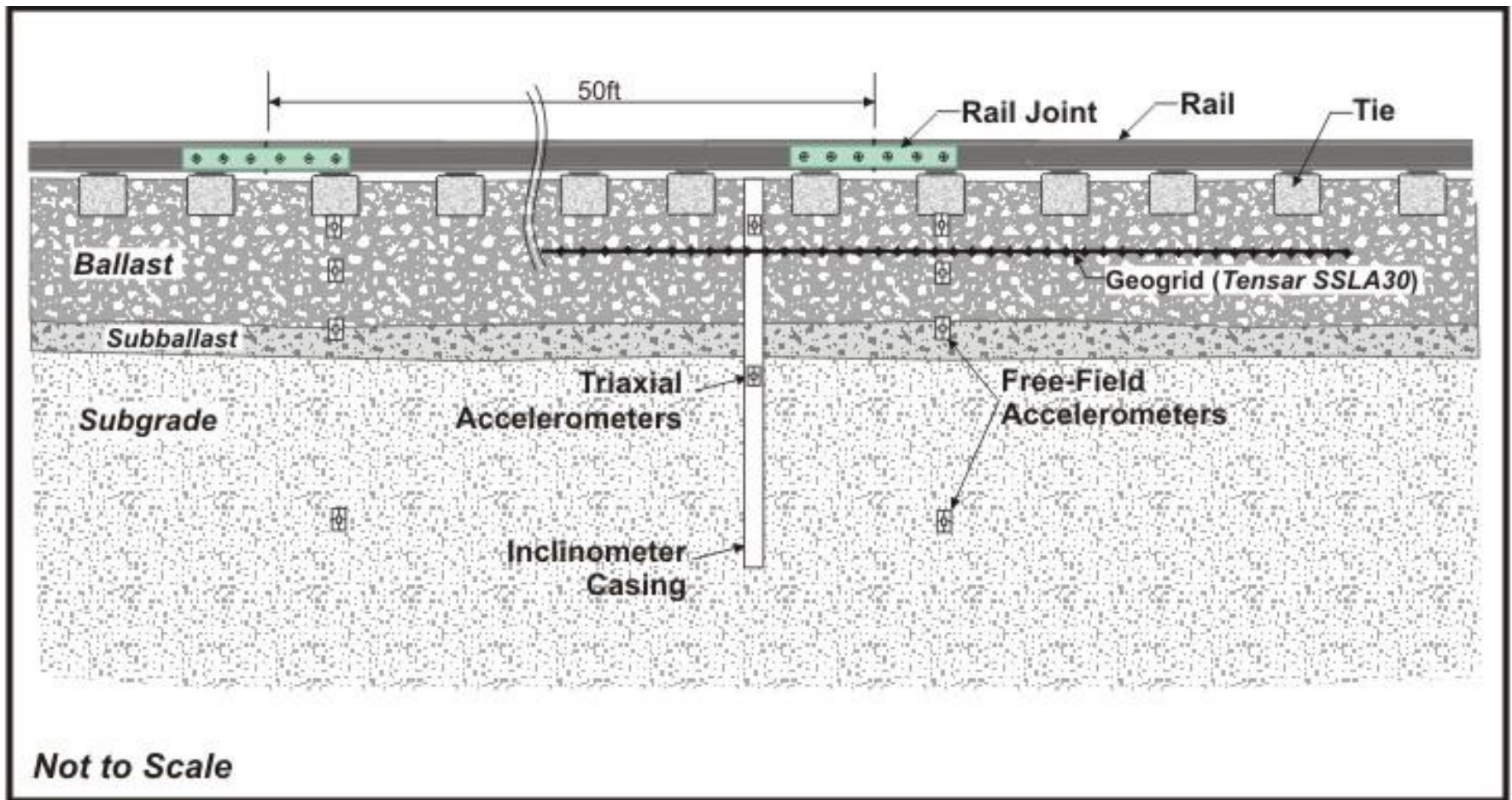
Reinforcement in  
Lower Subballast





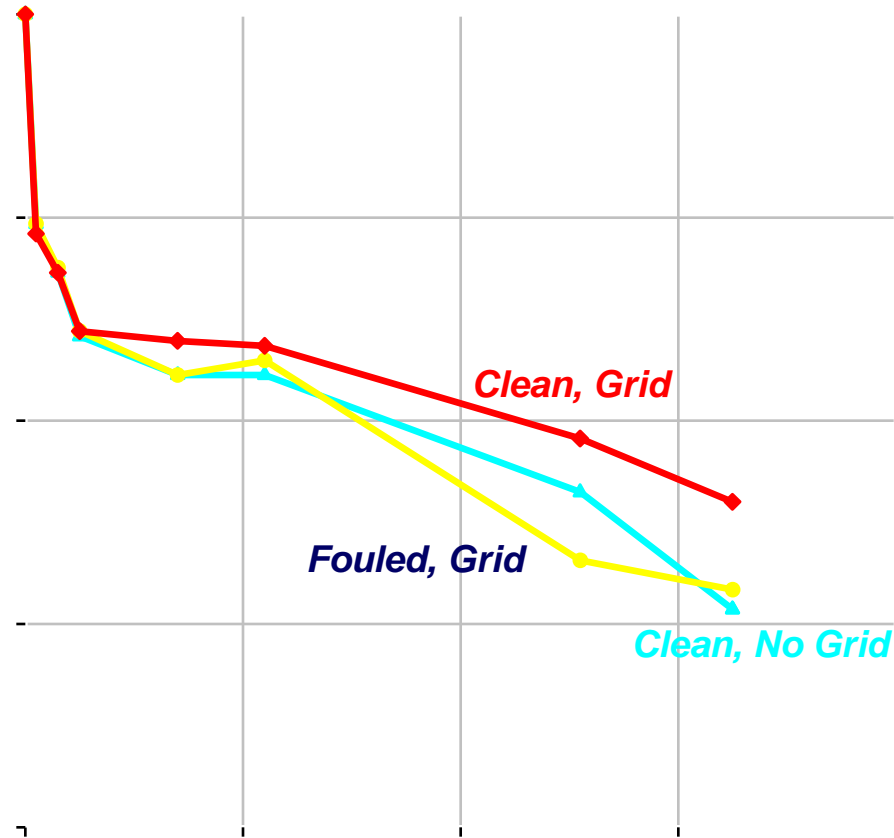


# Test Section



# Reinforced Ballast – TOR Settlement

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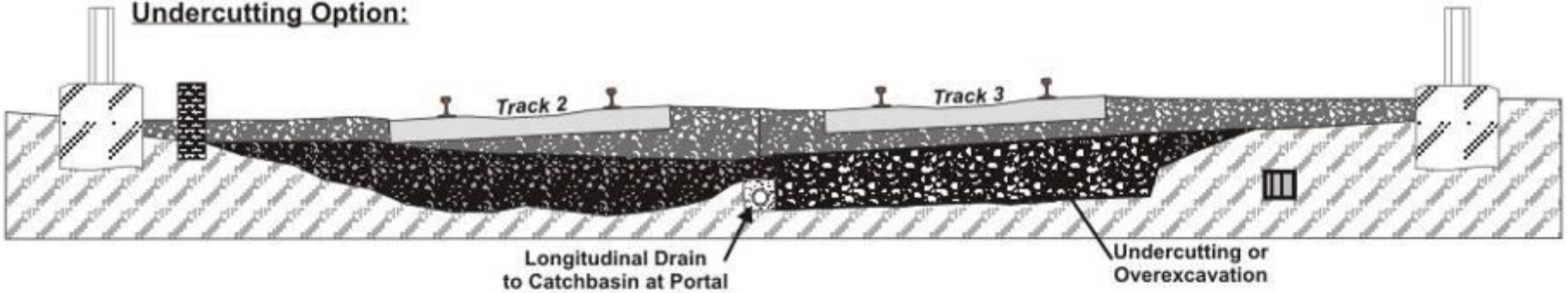
# Chemical Grouting



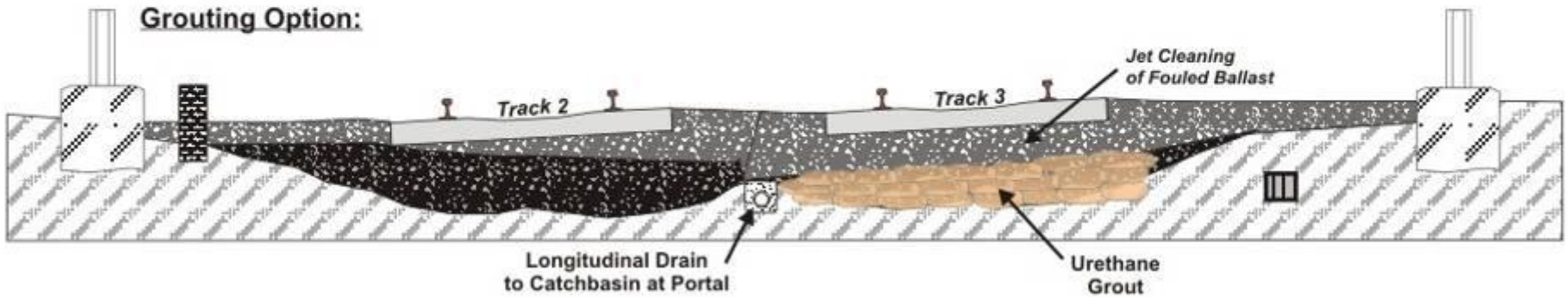
**Presumed Existing Conditions:**



**Undercutting Option:**



**Grouting Option:**



# References for Further Information

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- ◆ *Li, D., Sussmann, T. and Selig, E. (1996). "Procedure for Railway Track Granular Layer Thickness Determination," Association of American Railroads, Report Number R-898, Oct.*
- ◆ *Li, D., Selig, E. and Chrismer, S. (1996). "Methods for Railway Track Granular Layer Thickness Design", AAR TD96-006, Feb.*
- ◆ *Chang, C., Adegoke, C. and Selig, E. (1980). "GEOTRACK Model for Railroad Track Performance", Journal of the Geotechnical Engineering Division, ASCE, Nov.*
- ◆ *Selig, E. and Waters, J. (1994). Track Geotechnology and Substructure Management. Thomas Telford Services Ltd., London.*
- ◆ *Brown, S. and Selig, E. (1991). "The Design of Pavement and Rail Track Foundations." Cyclic Loading of Soil: from Theory to Design, Eds. M.P. O'Reilly and S.F. Brown, Van Nostrand Reinhold, New York, pp. 249-305.*

